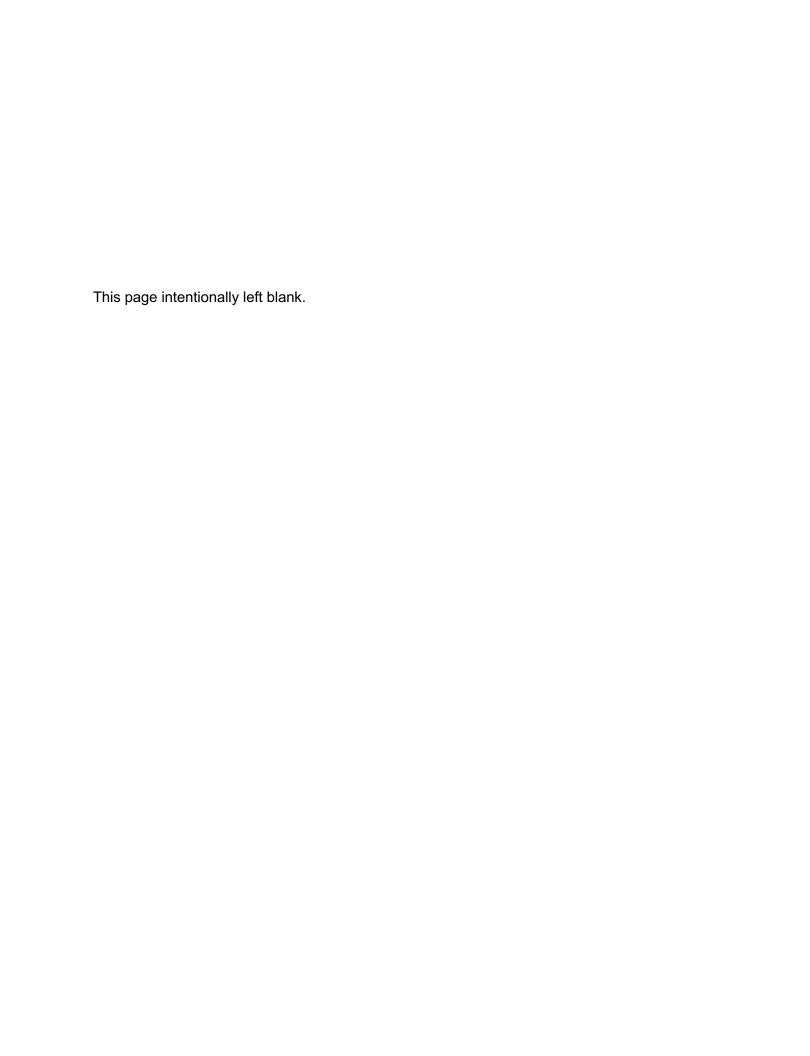
J-1 Local Transportation Analysis



CITY OF ENCINITAS Torrey Crest 30 Homes (MULTI-004309-2021) 1220-1240 Melba Rd and 1190 Island View Ln October 9, 2023

Draft Local Transportation Analysis

Prepared by Justin Rasas (RCE 60690), a principal with:



Job #2015

Table of Contents

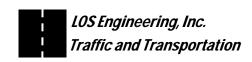
Exe	cutive Summary	…iv
1.0	Introduction	1
2.0	Transportation Analysis Methodology	4
2.1	Study Area	4
2.2	Study Scenarios	
2.3	Traffic Analysis Methodology	4
2	.3.1 Intersections	5
2	.3.2 Street Segments	5
2.4	General Plan Policy Compliance	6
3.0	Existing Conditions	7
3.1	Existing Street System	7
3.2	Existing Traffic Volumes and LOS Analyses	9
4.0	Project Description	11
4.1	Project Traffic Generation	11
4.2	Project Access	
4.3	Project Distribution and Assignment	
5.0	Existing + Project Conditions	14
6.0	Conclusion	
7.0	References	17
1 ! - 1		
LIST (of Figures	
	1: Project Location	
	2: Site Plan	
Figure	3: Existing Roadway Conditions	8
_	4: Existing Volumes	
	5: Project Distribution	
_	6: Project Assignment	
Figure	7: Existing + Project Volumes	15

List of Tables

Table 1: Intersection Level of Service Definitions (6 th Edition HCM)	5
Table 2: Street Segment Daily Capacity and LOS (City of Encinitas)	
Table 3: City of Encinitas General Plan Circuilation Element Policies 1.2 and 1.3	
Table 4: Balour Dr between Encinitas Blvd and Melba Rd Comparison	
Table 5: Existing Intersection Operations	
Table 6: Project Traffic Generation	
Table 7: Existing + Project Intersection Operations	
\mathcal{E} \mathcal{I}	

Appendices

Appendix A	Excerpts from City of Encinitas General Plan
Appendix B	Excerpts from City of Encinitas General Plan on General Plan Policy
Appendix C	Excerpts from City of Encinitas Circulation Plan
Appendix D	
Appendix E	
Appendix G	Existing Intersection LOS Worksheets
Appendix H	Existing + Project Intersection LOS Worksheets
	· · · · · · · · · · · · · · · · · · ·



Executive Summary Torrey Crest 30 Homes

This Local Transportation Analysis (LTA) determines if the proposed project would conflict with the City of Encinitas General Plan Circulation Element Policy 1.3 for the study intersections in the vicinity of the project. The proposed project is a subdivision with 30-homes. The project is located at 1220-1240 Melba Rd and 1190 Island View Ln in Encinitas, California. The site has six existing homes; however, only three were occupied during the Notice of Preparation in June 2022. Project sole access is from Melba Road.

This analysis is based on the local San Diego Institute of Transportation Engineers (ITE) Guidelines for Traffic Impact Studies in the San Diego Region, May 2019 traffic analysis criteria. The project traffic generation was calculated using the SANDAG trip rates from the Brief Guide of Vehicular Traffic Generation Rates for the San Diego Region, April 2002. The project site has active uses creating traffic; therefore, a traffic credit was applied because the existing uses will be replaced by the project. The trip credit was applied for three homes that were occupied during the Notice of Preparation in June 2022. The net change in project trip generation is calculated at 270 ADT, 21 AM peak hour trips (6 inbound and 15 outbound), and 27 PM peak hour trips (19 inbound and 8 outbound).

Traffic data was collected on Thursday August 26, 2021. Surrounding schools were confirmed to be in session and there were no stay-at-home orders during the data collection. Even though surrounding schools were in session, a comparison was made between available historical City segment data for a nearby location of Balour Dr between Encinitas Blvd and Melba Rd. Current data from 8/31/21 is 3.6% higher than historical data from 6/2/2015; therefore, the existing counts were used without adjustments. The findings by study scenario are summarized below.

- 1) Under Existing Conditions, the study intersections were calculated to operate at LOS C or better.
- 2) Under Existing plus Project Conditions, the study intersections were calculated to operate at LOS C or better. The addition of project traffic to the study intersections does not conflict with the City of Encinitas General Plan Circulation Element Policy 1.3; therefore, off-site roadway improvements are not required. However, the project applicant will be required to pay City traffic mitigation fees for City roadway improvements.

1.0 Introduction

The purpose of this Local Transportation Analysis is to determines if the proposed project traffic from 30 homes would conflict with the City of Encinitas General Plan Circulation Element Policy 1.3 for the study intersections in the vicinity of the project. The project is located at 1220-1240 Melba Rd and 1190 Island View Ln in Encinitas, California. The site has six existing homes; however, only three were occupied during the Notice of Preparation in June 2022. Project sole access is from Melba Road. The regional location of the project is shown in Figure 1. A site plan is shown in Figure 2.

This report describes the existing roadway network in the vicinity of the project site and includes a review of the existing and proposed activities for weekday peak AM and PM periods when the project is completed. The format of this study includes the following chapters:

- 1.0 Introduction
- Transportation Analysis Methodology 2.0
- **Existing Conditions** 3.0
- **Project Description** 4.0
- 5.0 Existing + Project Conditions
- Conclusion 6.0
- 7.0 References

Figure 1: Project Location

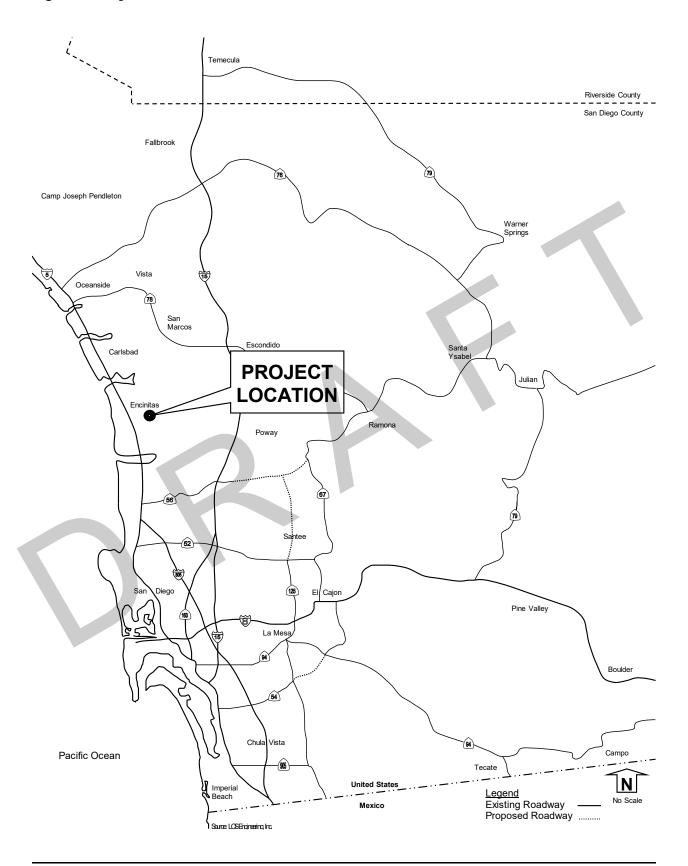
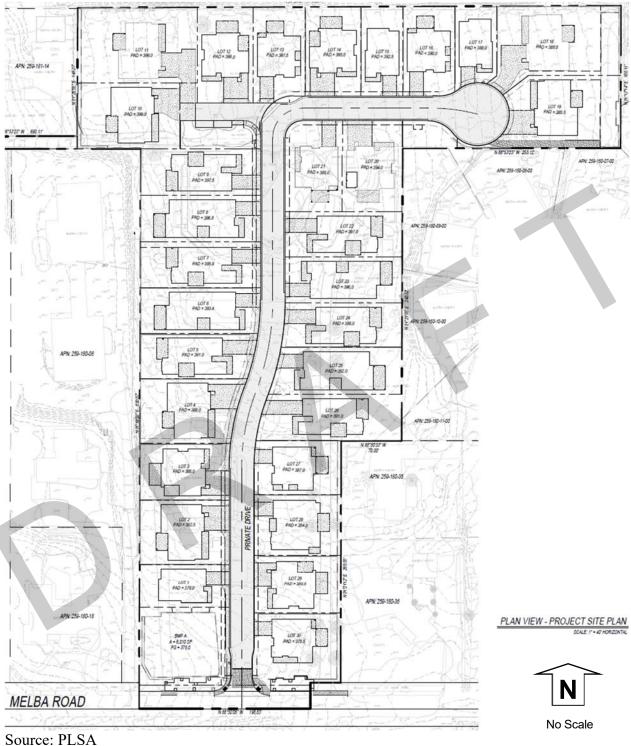


Figure 2: Site Plan



2.0 Transportation Analysis Methodology

A Local Transportation Analysis (LTA) is provided to fulfill the Encinitas Municipal Code 23.08.060 requirement of a traffic analysis for projects with more than 2,000 square feet of building area and/or any residential project with five or more units.

This traffic study was prepared based on direction from City staff on the study intersections and roadway segments; the scenarios to be analyzed; and the methods required for analysis. The criteria for each of these parameters are included herein.

2.1 Study Area

The following intersections were analyzed in this study:

- 1) Melba Rd/Balour Dr (un-signalized)
- 2) Melba Rd/Project Access (future intersection)
- 3) Melba Rd/Crest Dr (un-signalized)

The following street segment had data collected for this study:

1) Melba Rd from Balour Dr to Crest Dr

2.2 Study Scenarios

The number of scenarios to be analyzed is typically based on the size of the project. For this project, the following scenarios were included based on coordination with City staff:

- 1) Existing Conditions
- 2) Existing + Project Conditions

2.3 Traffic Analysis Methodology

The traffic analyses prepared for this study were based on the *Highway Capacity Manual* (HCM) operations analysis using Level of Service (LOS) evaluation criteria. The operating conditions of the study intersections and street segments were measured using the HCM LOS designations, which ranges from A through F. LOS A represents the best operating condition and LOS F denotes the worst operating condition.

2.3.1 Intersections

The study intersections were analyzed based on the **operational analysis** outlined in the 6th Edition HCM. This process defines LOS in terms of **average control delay** per vehicle, which is measured in seconds. LOS at the intersections were calculated using the computer software program Synchro 10 (Trafficware Corporation). The 6th Edition HCM LOS for the range of delay by seconds for intersections is shown in **Table 1**.

TABLE 1: INTERSECTION LEVEL OF SERVICE DEFINITIONS (6TH EDITION HCM)

TABLE II. INTEROCOTION LEVEL OF SERVICE DEFINITIONS (O EDITIONATION)										
Level of Service	Un-Signalized Control Delay	Signalized Control Delay								
	for TWSC, AWSC, and Roundabout	(sec/veh where v/c ≤ 1)								
	(sec/veh where $v/c \le 1$)									
Α	0-10	<u>≤</u> 10								
В	> 10-15	> 10-20								
С	> 15-25	> 20-35								
D	> 25-35	> 35-55								
Е	> 35-50	> 55-80								
F	> 50	> 80								

Source: 6th Edition HCM. TWSC: Two Way Stop Control. AWSC: All Way Stop Control. For unsignalized intersections, the control delay is the worst movement delay in seconds/vehicle.

2.3.2 Street Segments

The roadway segment daily capacity based on the General Plan are summarized in **Table 2**. The City of Encinitas General Plan segment capacities are included in **Appendix A**.

TABLE 2: STREET SEGMENT DAILY CAPACITY AND LOS (CITY OF ENCINITAS)

	Facility Type	Number of Lanes	LOS C	LOS D	LOS E
	Prime Arterial	6	<46,000	<51,200	<57,000
Prime	Arterial – Augmented	6	<53,000	<60,000	<66,000
	Major Roadway	4	<28,200	<31,600	<35,200
Major I	Roadway - Augmented	4+	<36,300	<41,000	<45,400
C	Collector Roadway	4	<26,000	<29,200	<32,400
Local I	Roadway - Augmented	2+	<16,000	<18,000	<20,000
	Local Roadway	2	<11,200	<12,600	<14,000

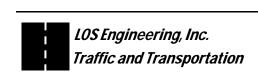
Source: City of Encinitas Public Road Standards April 1991.

2.4 General Plan Policy Compliance

The City of Encinitas General Plan Circulation Element defines traffic growth policies to promote an adequate roadway circulation system. This LTA determines if the project traffic would conflict with Policy 1.3. The Highway Capacity Manual Level of Service is used to determine compliance with Policy 1.3. If a project's traffic causes a conflict with Policy 1.3 based on the criteria shown in **Table 3**, then an overriding public need should be demonstrated if there is no existing alternative (General Plan Policy excerpts included in **Appendix B**).

TABLE 3: CITY OF ENCINITAS GENERAL PLAN CIRCUILATION ELEMENT POLICIES 1.2 AND 1.3

Circulation Element Policy	Application	Conflict Criteria
Policy 1.3: Prohibit development which results in Level of Service E or F at any intersection unless no alternatives exist and an overriding public need can be demonstrated.	Intersections	If project traffic causes the LOS to degrade to E/F, then there is a policy conflict. There is no policy conflict if the pre-project condition is at LOS E/F.



3.0 Existing Conditions

This section describes the study area streets, peak hour volumes, and existing LOS.

3.1 Existing Street System

In the vicinity of the project, the following roadways were analyzed for this report and are described below. The General Plan street classifications are ultimate classifications. The existing roadway conditions are shown in **Figure 3**.

Melba Rd from Balour Dr to Crest Dr is constructed as a two (2) lane roadway with one travel lane in each direction. There is no sidewalk on the south side along this segment. There is a sidewalk on the north side of the roadway along this segment. Bike Sharrow pavement markings are included along this segment. The posted speed limit is 25 Miles Per Hour. This segment is not classified on the City of Encinitas Circulation Plan (Appendix C). Because Melba Rd is not listed in the City of Encinitas Circulation Plan, a level of service cannot be provided for this residential street. However, the intersections at the ends of this streets can provide an indication of the operations. SANDAG also notes in their Congestion Management Program (excerpt included in Appendix D):

"LOS are not applied to residential streets since their primary purpose is to serve abutting lots, not carry through traffic. LOS normally apply to roads carrying through traffic between major trip generates and attractors."

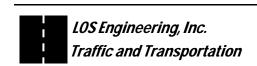
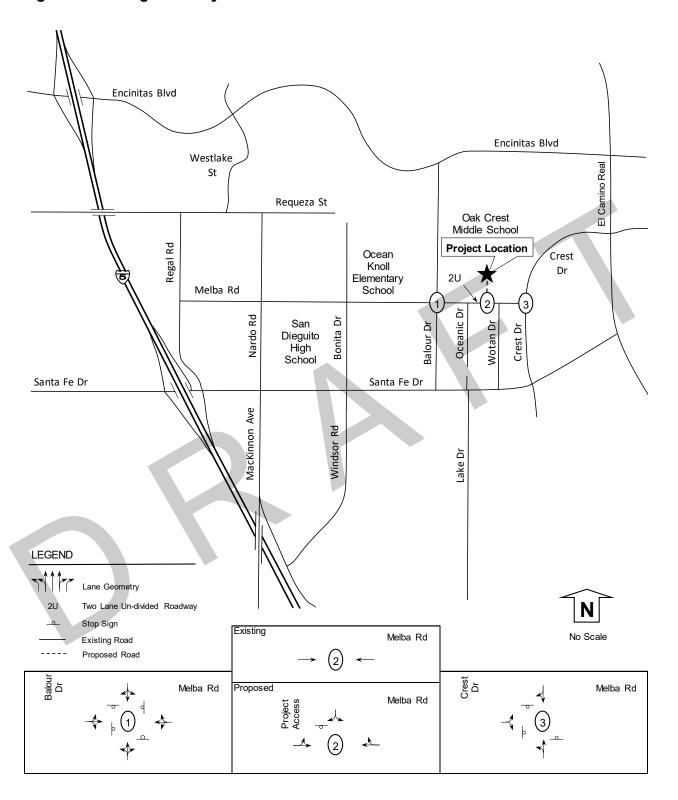


Figure 3: Existing Roadway Conditions



3.2 Existing Traffic Volumes and LOS Analyses

Intersection counts were collected between 7:00 AM to 9:00 AM for the AM commuter period and from 4:00 PM to 6:00 PM for the PM commuter period. Traffic data was collected on <u>Thursday August 26, 2021</u>. Surrounding schools were confirmed to be in session and there were no stay-at-home orders during the data collection. Counts are included in **Appendix E**. The intersection counts included:

- 1) Melba Rd/Balour Dr
- 2) Melba Rd/Project Access (through movement volumes from above intersection)
- 3) Melba Rd/Crest Dr

Daily counts (24 hour) were collected for the following segment:

1) Melba Rd from Balour Dr to Crest Dr

Even though surrounding schools were in session, a comparison was made between available historical City segment data for a nearby location of Balour Dr between Encinitas Blvd and Melba Rd (counts included in **Appendix F**). The City did not have any other nearby recent data. As shown in **Table 4**, current data from 8/31/21 is 3.6% higher than historical data from 6/2/2015; therefore, the existing counts were used without adjustments.

TABLE 4: BALOUR DR BETWEEN ENCINITAS BLVD AND MELBA RD COMPARISON

Count Date	Daily Volume
6/2/2015	7,988
8/31/2021	8,284
Percent Change:	3.6%

Table 5. Since the segment of Melba Rd is not classified, a LOS cannot be measured; therefore, a segment capacity table is not provided. Intersection LOS worksheets are included in **Appendix G**.

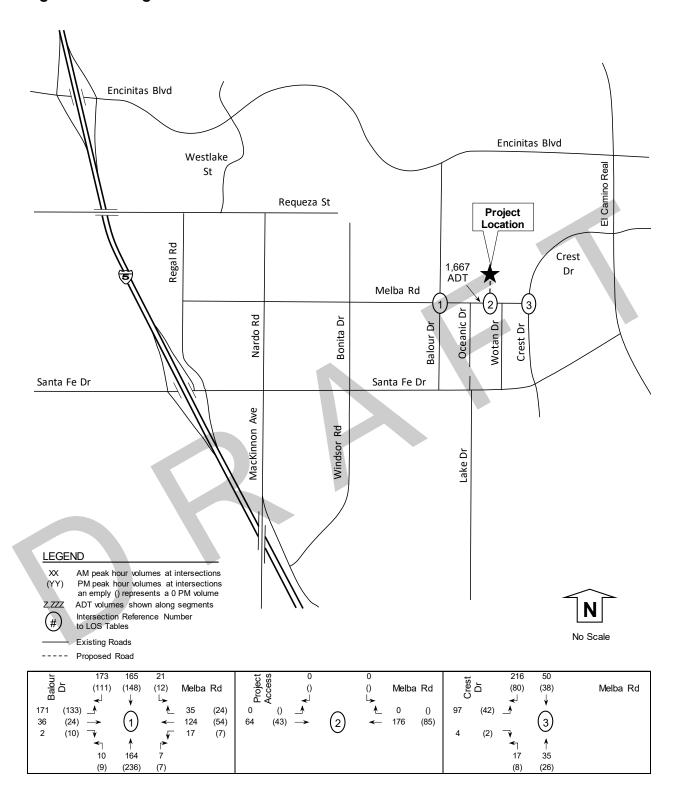
TABLE 5: EXISTING INTERSECTION OPERATIONS

Intersection and	Approach	Peak	Exist	ing
(Analysis) ¹		Hour	Delay ²	LOS ³
1) Melba Rd at	All	AM	16.1	С
Balour Dr (U)	All	PM	11.4	В
2) Melba Rd at	SB	AM	DNE	NA
Project Dwy (U)	SB	PM	DNE	NA
3) Melba Rd at	All	AM	9.5	A
Crest Dr (U)	All	PM	7.5	Α

Notes: 1) Intersection Analysis: (U) Unsignalized. 2) Delay:HCM Average Control Delay in seconds. 3) LOS:Level of Service.

Under Existing Conditions, the study intersections were calculated to operate at LOS C or better.

Figure 4: Existing Volumes



4.0 Project Description

The proposed project is a subdivision with 30-homes. The project is located at 1220-1240 Melba Rd and 1190 Island View Ln in Encinitas, California. The site has six existing homes; however, only three were occupied during the Notice of Preparation in June 2022. Project sole access is from Melba Road.

4.1 Project Traffic Generation

Project traffic generation was calculated using the SANDAG trip rates from the *Brief Guide of Vehicular Traffic Generation Rates for the San Diego Region*, April 2002. The project site has active uses creating traffic; therefore, a traffic credit was applied because the existing uses will be replaced by the project. The trip credit was applied for three homes that were occupied during the Notice of Preparation in June 2022. All existing homes on the site will be replaced by the project. The net change in project trip generation is calculated at 270 ADT, 21 AM peak hour trips (6 inbound and 15 outbound), and 27 PM peak hour trips (19 inbound and 8 outbound) as shown in **Table 6**.

TABLE 6: PROJECT TRAFFIC GENERATION

Proposed Land Hee							AM				PM	
Proposed Land Use Rate		Size &	Units	ADT	%	Split	IN	OUT	%	Split	IN	OUT
Existing Homes with Trip	Credit											
Single Family Homes	10 /DU	-3	DU	-30	8%	0.3 0.7	-1	-2	10%	0.7 0.3	-2	-1
Proposed Project												
Single Family Homes	10 /DU	30	DU	300	8%	0.3 0.7	7	17	10%	0.7 0.3	21	9
Net	Change:	27		270			6	15			19	8

Source: SANDAG Brief Guide of Vehicular Traffic Generation Rates for the San Diego Region, April 2002. ADT-Average Daily Traffic. Split-percent inbound and outbound. DU: Dwelling Unit. Spreadsheet rounding may result in +1 to above numbers.

4.2 Project Access

The project access is proposed from one driveway on Melba Rd.

4.3 Project Distribution and Assignment

Project trips were distributed to the adjacent roadway network using engineering judgement and factors such as proximity to Interstate 5 and local attractions. The project distribution also includes adding traffic to Wotan Dr and Crest Dr. The project distribution is shown in **Figure 5** with assignment of project volumes shown in **Figure 6**.

Figure 5: Project Distribution

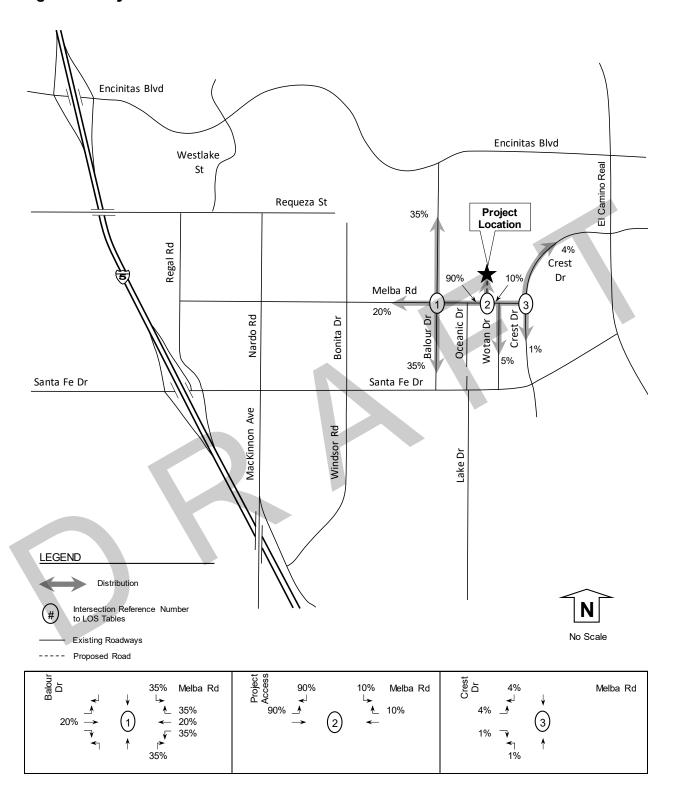
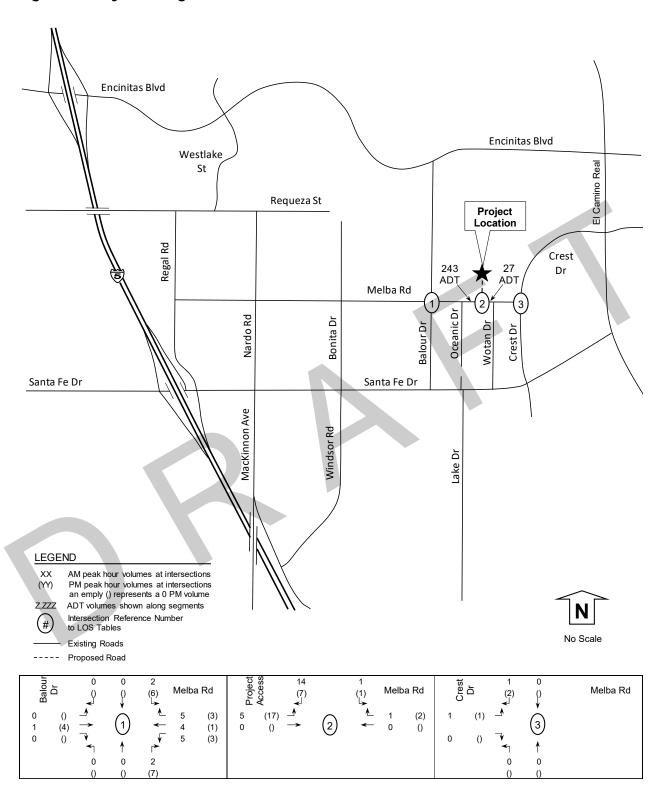


Figure 6: Project Assignment



5.0 Existing + Project Conditions

This scenario represents the addition of project traffic onto Existing volumes. The peak hour intersection volumes and daily traffic volumes are shown in **Figure 7**.

The LOS calculated for the intersections are shown in **Table 7.** Since the segment of Melba Rd is not classified, a LOS cannot be measured; therefore, a segment capacity table is not provided. Intersection LOS worksheets are included in **Appendix H**.

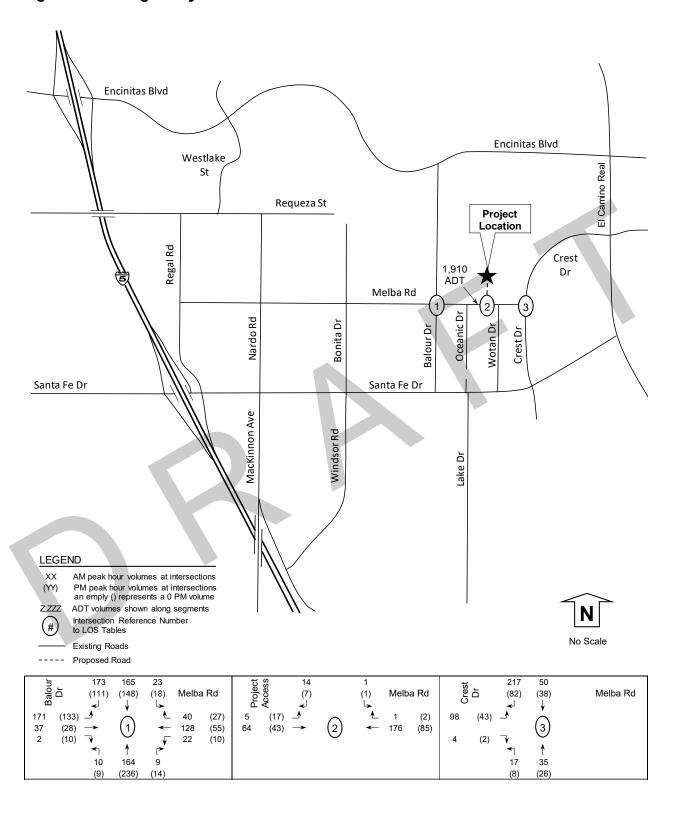
TABLE 7: EXISTING + PROJECT INTERSECTION OPERATIONS

Intersection	Approach Peak		Exi	sting	E	Existing + Project			
and (Analysis) ¹	Hour		Delay ²	LOS ³	Delay ²	LOS ³	GP Conflict? ⁴		
1) Melba Rd at	All	AM	16.1	С	16.8	С	No		
Balour Dr (U)	All	PM	11.4	В	11.7	В	No		
2) Melba Rd at	SB	AM	DNE	NA	9.6	Α	No		
Project Dwy (U)	SB	PM	DNE	NA	9.0	Α	No		
3) Melba Rd at	All	AM	9.5	Α	9.5	Α	No		
Crest Dr (U)	All	PM	7.5	Α	7.5	Α	No		

Notes: 1) Intersection Analysis - (S) Signalized, (U) Unsignalized. 2) Delay - HCM Average Control Delay in sec. 3) LOS: Level of Service. 4) General Plan Circulation Element Policy 1.3 conflict if project traffic causes an LOS E/F. DNE: Does Not Exist. NA: Not Applicable.

Under Existing plus Project Conditions, the study intersections were calculated to operate at LOS C or better. The addition of project traffic to the study intersections does not conflict with the City of Encinitas General Plan Circulation Element Policy 1.3; therefore, off-site roadway improvements are not required. However, the project applicant will be required to pay City traffic mitigation fees for City roadway improvements.

Figure 7: Existing + Project Volumes



6.0 Conclusion

This Local Transportation Analysis (LTA) determines if the proposed project would conflict with the City of Encinitas General Plan Circulation Element Policy 1.3 for the study intersections in the vicinity of the project. The proposed project is a subdivision with 30-homes. The project is located at 1220-1240 Melba Rd and 1190 Island View Ln in Encinitas, California. The site has six existing homes; however, only three were occupied during the Notice of Preparation in June 2022. Project sole access is from Melba Road.

This analysis is based on the local San Diego Institute of Transportation Engineers (ITE) Guidelines for Traffic Impact Studies in the San Diego Region, May 2019 traffic analysis criteria. The project traffic generation was calculated using the SANDAG trip rates from the Brief Guide of Vehicular Traffic Generation Rates for the San Diego Region, April 2002. The project site has active uses creating traffic; therefore, a traffic credit was applied because the existing uses will be replaced by the project. The trip credit was applied for three homes that were occupied during the Notice of Preparation in June 2022. All existing homes on the site will be replaced by the project. The net change in project trip generation is calculated at 270 ADT, 21 AM peak hour trips (6 inbound and 15 outbound), and 27 PM peak hour trips (19 inbound and 8 outbound).

Traffic data was collected on Thursday August 26, 2021. Surrounding schools were confirmed to be in session and there were no stay-at-home orders during the data collection. Even though surrounding schools were in session, a comparison was made between available historical City segment data for a nearby location of Balour Dr between Encinitas Blvd and Melba Rd. Current data from 8/31/21 is 3.6% higher than historical data from 6/2/2015; therefore, the existing counts were used without adjustments. The findings by study scenario are summarized below.

- 1) Under Existing Conditions, the study intersections were calculated to operate at LOS C or better.
- 2) Under Existing plus Project Conditions, the study intersections were calculated to operate at LOS C or better. The addition of project traffic to the study intersections does not conflict with the City of Encinitas General Plan Circulation Element Policy 1.3; therefore, off-site roadway improvements are not required. However, the project applicant will be required to pay City traffic mitigation fees for City roadway improvements.

7.0 References

City of Encinitas Circulation Element May 11, 1995.

Highway Capacity Manual (6th Edition).

San Diego Institute of Transportation Engineers (ITE). May 2019. *Guidelines for Transportation Impact Studies in the San Diego Region*.

San Diego Association of Governments (SANDAG). April 2002. Brief Guide of Vehicular Traffic Generation Rates for the San Diego Region.

Trafficware Corporation. Synchro 11.0 computer software.



Appendix A

Excerpts from City of Encinitas General Plan



THE CITY OF ENCINITAS CALIFORNIA

PUBLIC ROAD STANDARDS

April, 1991

TABLE 2
GENERAL PLAN CIRCULATION ELEMENT
ROADWAY CAPACITY STANDARDS *

Facility Type	# of	AD	T Capacity	
ractificy Type	Lanes	LOS C	LOS D	LOS E
FREEWAY	 6	108,00	120,000	135,000
	8	145,000	160,000	175,000
	10	175,000	195,000	215,000
Prime Arterial	6	46,000	51,200	57,000
 Prime Arterial-Augmented	. 6	53,000	60,000	66,000
 Major Roadway	4	28,200	31,600	35,200
 Major Roadway-Augmented	4+	36,300	41,000	45,400
 Collector Roadway	4	26,000	29,200	32,400
Local Roadway-Augmented	2+	16,000	18,000	20,000
 Local Roadway	2	11,200	12,600	14,000

NOTE: 1. Capacity means the maximum volume for the stated level of service.

 The above Standards are not applicable to noncirculation element roadways.

^{*} From City of Encinitas General Plan Circulation Element.

Appendix B

Excerpts from City of Encinitas General Plan on General Plan Policy



CIRCULATION ELEMENT

CITY OF ENCINITAS GENERAL PLAN

As Amended 8/25/93, 1/12/94 9/21/94 and 5/11/95



CIRCULATION ELEMENT GOALS AND POLICIES

The following goals and policies included in this Element address a wide range of issues concerning circulation in and through the City. More efficient movement of traffic on existing roadways, the establishment of standards for future roads, provision of other forms of transit, preservation of scenic highways, and improved coastal access are the major areas of concern of the following goals and policies.

Safe,
Convenient,
and Efficient
Transportation
System

The following goal and supporting policies emphasize the need to maintain a transportation system that is capable of handling the existing and projected traffic loads in the City. To achieve this end, a number of policies have been adopted that call for more efficient use of existing roadways by employing measures that improve the movement of traffic.

GOAL 1: Encinitas should have a transportation system that is safe, convenient and efficient, and sensitive to and compatible with surrounding community character. (Coastal Act/30252)

POLICY 1.1: Ensure that the arterial circulation system provides adequate connections across the freeway for convenient circulation and rapid emergency access.

POLICY 1.2: Endeavor to maintain Level of Service C as a basic design guideline for the local system of roadways understanding that the guideline may not be attainable in all cases.

POLICY 1.3: Prohibit development which results in Level of Service E or F at any intersection unless no alternatives exist and an overriding public need can be demonstrated.

POLICY 1.4: Require, where feasible, interconnecting offstreet pedestrian and vehicular circulation between adjacent commercial and office land uses. This policy should be required along major transportation corridors to minimize traffic conflicts associated with pedestrian and vehicular movement to and from these properties. (Coastal Act/30252)

POLICY 1.5: Promote maximum utilization or expansion of existing freeways and prime arterials as an alternative to new freeway or highway construction. Encourage new and/or proposed freeway construction to be outside the Encinitas sphere of influence boundaries.

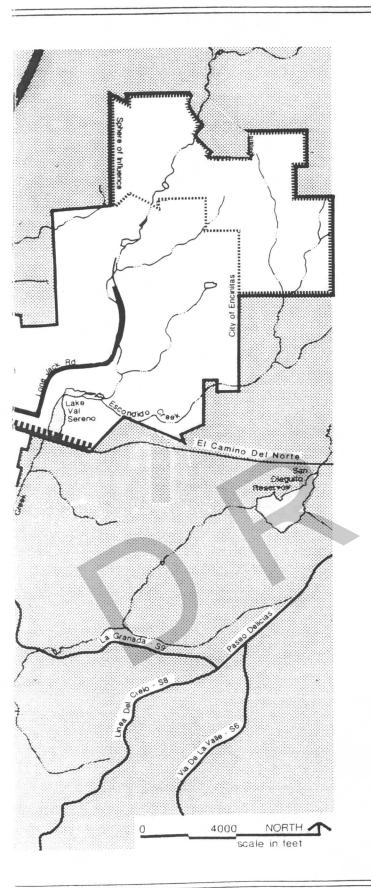


Appendix C

Excerpts from City of Encinitas Circulation Plan







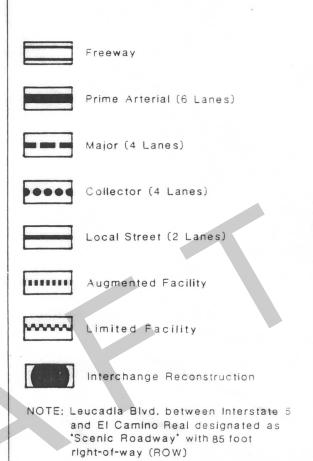


Figure 2 Circulation Plan



Appendix D

Excerpt from SANDAG CMP



FINAL 2008 CONGESTION MANAGEMENT PROGRAM UPDATE

BOARD OF DIRECTORS



The 18 cities and county government are SANDAG serving as the forum for regional decision-making. SANDAG builds consensus; plans, engineers, and builds public transit; makes strategic plans; obtains and allocates resources; and provides information on a broad range of topics pertinent to the region's quality of life.

CHAIR

Hon. Mary Teresa Sessom

FIRST VICE CHAIR

Hon. Lori Holt Pfeiler

SECOND VICE CHAIR Hon. Jerome Stocks **EXECUTIVE DIRECTOR**

Gary L. Gallegos

CITY OF CARLSBAD

Hon. Matt Hall, Councilmember (A) Hon. Bud Lewis, Mayor (A) Hon. Ann Kulchin, Mayor Pro Tem

CITY OF CHULA VISTA

Hon. Cheryl Cox, Mayor (A) Hon. Jerry Rindone, Deputy Mayor (A) Hon. John McCann, Councilmember

CITY OF CORONADO

Hon. Phil Monroe, Councilmember (A) Hon. Carrie Downey, Mayor Pro Tem (A) Hon. Al Ovrom, Councilmember

CITY OF DEL MAR

Hon. Crystal Crawford, Deputy Mayor (A) Hon. David Druker, Mayor (A) Hon. Richard Earnest, Councilmember

CITY OF EL CAJON

Hon. Mark Lewis, Mayor (A) Hon. Jillian Hanson-Cox, Councilmember

CITY OF ENCINITAS

Hon. Jerome Stocks, Mayor (A) Hon. Teresa Barth, Councilmember

CITY OF ESCONDIDO

Hon. Lori Holt Pfeller, Mayor (A) Hon. Ed Gallo, Councilmember (A) Hon. Sam Abed, Mayor Pro Tem

CITY OF IMPERIAL BEACH

Hon. Jim Janney, Mayor (A) Hon. Patricia McCoy, Mayor Pro Tem (A) Hon. Mayda Winter, Councilmember

CITY OF LA MESA

Hon. Art Madrid, Mayor (A) Hon. Mark Arapostathis, Councilmember (A) Hon. David Allan, Councilmember

CITY OF LEMON GROVE

Hon. Mary Teresa Sessom, Mayor (A) Hon. Jerry Jones, Councilmember (A) Hon. Jerry Selby, Mayor Pro Tem

CITY OF NATIONAL CITY

Hon. Ron Morrison, Mayor (A) Hon. Frank Parra, Vice Mayor (A) Hon. Louie Natividad, Councilmember

CITY OF OCEANSIDE

Hon. Jim Wood, Mayor (A) Hon. Jerry Kern, Councilmember (A) Hon. Jack Feller, Councilmember

CITY OF POWAY

Hon. Mickey Cafagna, Mayor (A) Hon. Robert Emery, Deputy Mayor (A) Hon. Don Higginson, Councilmember

CITY OF SAN DIEGO

Hon. Jerry Sanders, Mayor Hon. Jim Madaffer, Council President Pro Tem (A) Hon. Anthony Young, Councilmember (A) Hon. Scott Peters, Council President (A) Hon. Toni Atkins, Councilmember (A) Hon. Ben Hueso, Councilmember

CITY OF SAN MARCOS

Hon. Jim Desmond, Mayor (A) Hon. Hal Martin, Vice Mayor (A) Hon. Rebecca Jones, Councilmember CITY OF SANTEE

Hon. Jack Dale, Councilmember (A) Hon. Hal Ryan, Councilmember (A) Hon. John Minto, Councilmember

CITY OF SOLANA BEACH

Hon. Lesa Heebner, Councilmember (A) Hon. Dave Roberts, Mayor (A) Hon. Mike Nichols, Deputy Mayor

CITY OF VISTA

Hon. Judy Ritter, Councilmember (A) Hon. Bob Campbell, Councilmember (A) Hon. Steve Gronke, Councilmember

COUNTY OF SAN DIEGO

(A) Hon. Greg Cox, Chairman
(A) Hon. Pam Slater-Price, Chair Pro Tem
(A) Hon. Ron Roberts, Supervisor
Hon. Dianne Jacob, Vice Chairwoman
(A) Hon. Bill Horn, Supervisor

IMPERIAL COUNTY

(Advisory Member) Hon. Victor Carrillo, Supervisor (A) Hon. David Ouzan, Councilmember

CALIFORNIA DEPARTMENT OF TRANSPORTATION

(Advisory Member) Will Kempton, Director (A) Pedro Orso-Delgado, District 11 Director

METROPOLITAN TRANSIT SYSTEM

(Advisory Member) Harry Mathis, Chairman (A) Hon. Jerry Rindone, Vice Chairman (A) Hon. Robert Emery, Chair Pro Tem

NORTH COUNTY TRANSIT DISTRICT

(Advisory Member) Hon. Ed Gallo, Chairman (A) Hon. Jerome Stocks, Planning Committee Chair (A) Hon. Chris Orlando, Monitoring Committee Chair

U.S. DEPARTMENT OF DEFENSE

(Advisory Member)
CAPT Steve Wirsching, USN, CEC,
Southwest Division Naval Facilities Engineering Command
(A) CAPT Robert Farley, USN, CEC
Southwest Division Naval Facilities Engineering Command

SAN DIEGO UNIFIED PORT DISTRICT

(Advisory Member) Laurie Black, Commissioner (A) Michael Najera, Commissioner

SAN DIEGO COUNTY WATER AUTHORITY

(Advisory Member) Marilyn Dailey, Commissioner (A) Mark Muir, Commissioner (A) Gary Croucher, Commissioner

SOUTHERN CALIFORNIA TRIBAL CHAIRMEN'S ASSOCIATION

(Advisory Member) Chairman Robert Smith (Pala), SCTCA Chair (A) Chairman Allen Lawson (San Pasqual)

MEXICO

(Advisory Member) Hon. Remedios Gómez-Amau Cónsul General of Mexico

As of July 16, 2008

Table D.2
Roadway Classifications, LOS, and ADT

Church Classification		Cross Sections*	LOS with ADT**							
Street Classification	Lanes	(approx.)	Α	В	C	D	E			
Expressway	6 lanes	102-160/122-200	30,000	42,000	60,000	70,000	80,000			
Prime Arterial	6 lanes	102-108/122-128	25,000	35,000	50,000	55,000	60,000			
Major Arterial	6 lanes	102/122	20,000	28,000	40,000	45,000	50,000			
Major Arterial	4 lanes	78-82/98-102	15,000	21,000	30,000	35,000	40,000			
Secondary Arterial/Collector	4 lanes	64-72/84-92	10,000	14,000	20,000	25,000	30,000			
Collector (no center lane) (continuous left-turn lane)	4 lanes 2 lanes	64/84 50/70	5,000	7,000	10,000	13,000	15,000			
Collector (no fronting property)	2 lanes	40/60	4,000	5,500	7,500	9,000	10,000			
Collector (commercial-industrial fronting)	2 lanes	50/70	2,500	3,500	5,000	6,500	8,000			
Collector (multi-family)	2 lanes	40/60	2,500	3,500	5,000	6,500	8,000			
Sub-Collector (single-family)	2 lanes	36/56	_		2,200	_	_			

LEGEND:

- * Curb-to-curb width (feet)/right of way width (feet): based upon the City of San Diego Street Design Manual and other jurisdictions within the San Diego region.
- ** Approximate recommended ADT based upon the City of San Diego Street Design Manual.

Notes:

- The volumes and the average daily level of service listed above are only intended as a general planning guideline.
- LOS are not applied to residential streets since their primary purpose is to serve abutting lots, not carry through traffic. LOS normally apply to roads carrying through traffic between major trip generators and attractors.

Not all mitigation measures can feasibly be "hard" (new lanes or new capacity) improvements. A sample mitigation measure might include financing toward a regional ITS (Intelligent Transportation System) project, such as improved or "dynamic" ramp metering with real-time delay information available to motorists. The information can be accessed on either home or in-vehicle computers, or even by telephone (each ramp could have its own phone number with delay information) so the motorist can make a driving decision long before she or he arrives at a congested on-ramp. This sample mitigation would allow a project applicant (especially with a relatively small project) to meet mitigation by paying into a regional ramp meter fee, providing the fee can be established in the near future. In identifying potential mitigation measures, the CMP *Toolbox of Mitigation Strategies* and any adopted Deficiency Plans in the study area also should be consulted.

Other mitigation measures may include Transportation Demand Management (TDM) recommendations – transit facilities, bike facilities, walkability, telecommuting, traffic rideshare programs, flex-time, carpool incentives, parking cash-out, etc. Additional mitigation measures may become acceptable as future technologies and policies evolve.

Appendix E

Count Data





Location: Encinitas N/S: **Balour Drive** E/W: Melba Road

Date: 8/26/2021 Day: THURSDAY Project # 143-21377

TURNING MOVEMENT COUNT

Count Period: 7:00 AM to 9:00 AM 7:15 AM to 8:15 AM Peak Hour:

Vehicle Counts

		alour Driv Iorthbour	-		alour Driv outhbour	-		Aelba Roa Eastboun	-		1elba Roa Vestboun		
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
7:00 AM	0	27	0	0	30	27	15	1	0	2	10	5	117
7:15 AM	0	22	0	0	39	66	37	4	0	2	40	4	214
7:30 AM	5	38	2	1	32	61	43	13	0	6	43	9	253
7:45 AM	3	47	4	4	39	27	45	7	1	5	28	10	220
8:00 AM	2	57	1	16	55	19	46	12	1	4	13	12	238
8:15 AM	0	22	0	4	45	12	18	7	0	3	9	5	125
8:30 AM	0	20	0	6	28	11	12	4	0	2	9	5	97
8:45 AM	0	27	2	3	23	8	14	6	1	2	7	2	95
TOTAL VOLUMES:	10	260	9	34	291	231	230	54	3	26	159	52	1359

AM Peak Hr Begins at:

	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
PEAK VOLUMES:	10	164	7	21	165	173	171	36	2	17	124	35	925
PEAK HR FACTOR:		0.754			0.855			0.886			0.759		0.914

Bicycle Counts

													_
	Ba	alour Driv	/e	В	alour Driv	re l	V	/lelba Roa	nd	N	⁄lelba Roa	ıd	
	N	lorthbour	nd	S	outhbour	nd		Eastbound	d	V	Vestboun	d	
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
7:00 AM	0	0	0	0	1	4	0	0	0	0	0	2	7
7:15 AM	0	0	0	0	0	0	0	0	0	1	2	0	3
7:30 AM	0	4	1	0	6	12	5	4	0	3	14	1	50
7:45 AM	2	2	0	0	3	1	21	10	1	0	9	2	51
8:00 AM	0	0	0	0	0	1	12	2	0	0	0	1	16
8:15 AM	0	0	0	0	1	0	2	2	0	0	0	0	5
8:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	1
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	2	6	1	0	11	19	40	18	1	4	25	6	133
			•	•							•		

	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
PEAK VOLUMES:	2	6	1	0	9	14	38	16	1	4	25	4	120

	Balour Drive	Balour Drive	Melba Road	Melba Road	
	North Leg	South Leg	East Leg	West Leg	TOTAL
7:00 AM	3	1	1	0	5
7:15 AM	6	1	1	0	8
7:30 AM	53	3	9	1	66
7:45 AM	148	9	15	0	172
8:00 AM	33	2	0	0	35
8:15 AM	3	0	0	0	3
8:30 AM	4	1	2	6	13
8:45 AM	1	1	1	0	3
TOTAL VOLUMES:	251	18	29	7	305

	North Leg	South Leg	East Leg	West Leg	TOTAL
PEAK VOLUMES:	240	15	25	1	281



Location: Encinitas N/S: Balour Drive E/W: Melba Road Date: 8/26/2021 Day: THURSDAY Project # 143-21377

TURNING MOVEMENT COUNT

Count Period: 4:00 PM to 6:00 PM Peak Hour: 4:45 PM to 5:45 PM

Vehicle Counts

		alour Driv Iorthbour	-		alour Driv outhbour			nelba Roa Eastboun			1elba Roa Vestboun		
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
4:00 PM	4	47	3	2	36	31	33	11	4	0	14	8	193
4:15 PM	1	45	0	1	41	27	42	8	0	1	14	3	183
4:30 PM	1	40	1	5	37	26	27	6	1	0	11	3	158
4:45 PM	3	59	1	1	43	32	34	4	2	1	20	5	205
5:00 PM	3	53	0	2	35	31	22	8	4	1	9	3	171
5:15 PM	2	59	4	6	35	25	42	5	2	4	11	10	205
5:30 PM	1	65	2	3	35	23	35	7	2	1	14	6	194
5:45 PM	3	52	4	0	33	24	33	6	7	2	18	3	185
TOTAL VOLUMES:	18	420	15	20	295	219	268	55	22	10	111	41	1494

PM Peak Hr Begins at: 445 PM

	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
PEAK VOLUMES:	9	236	7	12	148	111	133	24	10	7	54	24	775
-													
PEAK HR FACTOR:		0.926			0.891			0.852			0.817		0.945

Bicycle Counts

		alour Driv Iorthbour			alour Driv outhbour			Aelba Roa Eastboun			nelba Roa Vestboun		
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
4:00 PM	0	1	0	0	0	0	0	1	0	0	0	0	2
4:15 PM	0	0	0	0	0	0	1	3	0	0	0	0	4
4:30 PM	1	0	0	0	0	0	0	0	0	0	1	1	3
4:45 PM	0	2	0	0	1	2	0	1	0	0	0	0	6
5:00 PM	0	1	0	0	2	0	4	3	0	0	1	0	11
5:15 PM	0	0	0	0	0	0	0	0	1	0	3	0	4
5:30 PM	0	0	0	0	0	0	0	1	0	0	2	0	3
5:45 PM	0	0	0	0	0	0	3	0	0	0	0	0	3
TOTAL VOLUMES:	1	4	0	0	3	2	8	9	1	0	7	1	36

	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
PEAK VOLUMES:	0	3	0	0	3	2	4	5	1	0	6	0	24

	Balour Drive North Leg	Balour Drive South Leg	Melba Road East Leg	Melba Road West Leg	TOTAL
4:00 PM	1	2	2	3	8
4:15 PM	0	1	0	0	1
4:30 PM	1	0	0	0	1
4:45 PM	6	2	1	0	9
5:00 PM	1	1	1	0	3
5:15 PM	2	0	3	0	5
5:30 PM	3	0	0	0	3
5:45 PM	8	0	0	0	8
TOTAL VOLUMES:	22	6	7	3	38

	North Leg	South Leg	East Leg	West Leg	TOTAL
PEAK VOLUMES:	12	3	5	0	20



Location: Encinitas N/S: Crest Drive E/W: Melba Road Date: 8/26/2021 Day: THURSDAY Project # 143-21377

TURNING MOVEMENT COUNT

Count Period: 7:00 AM to 9:00 AM
Peak Hour: 7:15 AM to 8:15 AM

Vehicle Counts

	(Crest Driv	е	(Crest Driv	е	N	∕lelba Roa	ıd	N	1elba Roa	ıd	
	N	Iorthbour	nd	S	outhbour	nd	I	Eastboun	d	٧	√estboun	d	
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
7:00 AM	2	1	0	0	3	10	2	0	3	0	0	0	21
7:15 AM	1	2	0	0	8	57	6	0	0	0	0	0	74
7:30 AM	9	11	0	0	12	85	24	0	1	0	0	0	142
7:45 AM	6	17	0	0	12	57	34	0	2	0	0	0	128
8:00 AM	1	5	0	0	18	17	33	0	1	0	0	0	75
8:15 AM	0	3	0	0	6	17	7	0	2	0	0	Ŏ	35
8:30 AM	1	7	0	1	7	10	8	0	0	0	0	0	34
8:45 AM	0	3	0	0	6	8	14	0	1	0	0	0	32
TOTAL VOLUMES:	20	49	0	1	72	261	128	0	10	0	0	0	541

AM Peak Hr Begins at: 715 AM

	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
PEAK VOLUMES:	17	35	0	0	50	216	97	0	4	0	0	0	419

PEAK HR FACTOR: 0.565 0.686 0.701 0.000 0.738

Bicycle Counts

	(rest Driv	/P	(Crest Driv	ρ ,	Λ	/lelba Roa	ad	N.	1elba Roa	ıd	
		orthbou			outhbour			Eastboun			Vestboun		
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
7:00 AM	0	0	0	0	0	1	0	0	0	0	0	0	1
7:15 AM	1	0	0	0	0	4	0	0	0	0	0	0	5
7:30 AM	0	0	0	0	0	11	0	0	0	0	0	0	11
7:45 AM	3	0	0	0	1	12	6	0	0	0	0	0	22
8:00 AM	0	0	0	0	0	1	1	0	2	0	0	0	4
8:15 AM	0	0	0	0	0	0	2	0	0	0	0	0	2
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES:	4	0	0	0	1	29	9	0	2	0	0	0	45
		•		•	•				•	•	•	•	•

		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
PEAK VO	DLUMES:	4	0	0	0	1	28	7	0	2	0	0	0	42

	Crest Drive	Crest Drive	Melba Road	Melba Road	
	North Leg	South Leg	East Leg	West Leg	TOTAL
7:00 AM	1	0	0	1	2
7:15 AM	1	0	0	0	1
7:30 AM	0	0	0	4	4
7:45 AM	8	0	0	0	8
8:00 AM	0	0	0	0	0
8:15 AM	2	0	0	2	4
8:30 AM	0	0	0	2	2
8:45 AM	0	0	0	1	1
TOTAL VOLUMES:	12	0	0	10	22

	North Leg	South Leg	East Leg	West Leg	TOTAL
PEAK VOLUMES:	9	0	0	4	13



Location: Encinitas N/S: Crest Drive E/W: Melba Road Date: 8/26/2021 Day: THURSDAY Project # 143-21377

TURNING MOVEMENT COUNT

Count Period: 4:00 PM to 6:00 PM Peak Hour: 5:00 PM to 6:00 PM

Vehicle Counts

		Crest Driv Iorthbour	-		Crest Driv			Лelba Roa Eastboun			1elba Roa Vestbour		
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
4:00 PM	2	6	0	0	9	16	14	0	0	0	0	0	47
4:15 PM	1	4	0	0	4	17	8	0	0	0	0	0	34
4:30 PM	2	7	0	0	6	12	10	0	0	0	0	0	37
4:45 PM	1	1	0	0	4	27	9	0	1	0	0	0	43
5:00 PM	2	5	0	0	10	15	9	0	0	0	0	0	41
5:15 PM	1	10	0	0	7	19	12	0	2	0	0	0	51
5:30 PM	4	8	0	0	11	20	10	0	0	0	0	0	53
5:45 PM	1	3	0	0	10	26	11	0	0	0	0	0	51
TOTAL VOLUMES:	14	44	0	0	61	152	83	0	3	0	0	0	357

PM Peak Hr Begins at: 500 PM

	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
PEAK VOLUMES:	8	26	0	0	38	80	42	0	2	0	0	0	196

PEAK HR FACTOR: 0.708 0.819 0.786 0.000 0.925

Bicycle Counts

		Crest Driv Iorthbour			Crest Driv outhbour			Aelba Roa Eastboun			1elba Roa Vestboun		
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
4:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	1
4:15 PM	0	1	0	0	0	1	1	0	0	0	0	0	3
4:30 PM	0	1	0	0	1	1	0	0	0	0	0	0	3
4:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	1
5:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	1
5:15 PM	0	0	0	0	0	1	1	0	0	0	0	0	2
5:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	1
TOTAL VOLUMES:	0	2	0	0	2	4	5	0	0	0	0	0	13

	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
PEAK VOLUMES:	0	0	0	0	1	2	2	0	0	0	0	0	5

	Crest Drive	Crest Drive	Melba Road	Melba Road	
	North Leg	South Leg	East Leg	West Leg	TOTAL
4:00 PM	0	0	0	0	0
4:15 PM	0	1	0	2	3
4:30 PM	0	0	0	1	1
4:45 PM	0	0	0	0	0
5:00 PM	0	0	0	1	1
5:15 PM	0	0	0	0	0
5:30 PM	0	0	0	0	0
5:45 PM	0	0	0	0	0
TOTAL VOLUMES:	0	1	0	4	5

	North Leg	South Leg	East Leg	West Leg	TOTAL
PEAK VOLUMES:	0	0	0	1	1

ECN001

Site Code: 143-21377

Counts Unlimited, Inc. PO Box 1178

PO Box 1178 Corona, CA 92878 Phone: (951) 268-6268 email: counts@countsunlimited.com

City of Encinitas Melba Road B/ Oceanic Drive - Wotan Drive 24 Hour Directional Volume Count

Start	06-Sep-21	Easth	oound	Hour ⁻	Totals	West	tbound	Hour	Totals	Combine	ed Totals
Time	Mon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon
12:00		0	5			0	12				
12:15		0	6			1	16				
12:30		1	8			1	12				
12:45		0	7	1	26	1	12	3	52	4	78
01:00		Ö	8	•		0	4	· ·		·	
01:15		0	2			0	7				
01:30		1	8			0	18				
01:30		0	7	1	25	1	22	1	51	2	76
				'	23			ı	JI	2	70
02:00		0	5			1	37				
02:15		0	11			0	25				
02:30		0	35			0	24				
02:45		0	22	0	73	0	39	1	125	1	198
03:00		1	41			0	23				
03:15		0	32			0	22				
03:30		0	12			0	25				
03:45		1	6	2	91	0	17	0	87	2	178
04:00		0	18	_	0.	2	21		O.	_	
04:15		0	10			0	18				
04:30		1	9			0	13				
04.30		0		4	46		26	2	78	3	124
04.45		0	9	1	40	0	20	2	18	3	124
05:00			12			0	18				
05:15		1	10			1	19				
05:30		0	10	_		1	22			_	
05:45		1	7	2	39	3 2 2	20	5	79	7	118
06:00		1	8			2	18				
06:15		0	12			2	16				
06:30		0	8			3	14				
06:45		1	6	2	34	7	10	14	58	16	92
07:00		3	9			15	17				
07:15		5	5			59	9				
07:30		23	4			111	8				
07:45		39	3	70	21	69	8	254	42	324	63
08:00		44	4	70	21	30	3	254	42	324	03
00.00		44	4			30					
08:15		2 9	3 2			5	6				
08:30		9				14	8				
08:45		7	1	62	10	7	6	56	23	118	33
09:00		6 5 6	2			11	4				
09:15		5	1			8	2				
09:30			1			10	4				
09:45	1	10	3	27	7	6	2	35	12	62	19
10:00		2	4			9	2				
10:15		3	1			15	0				
10:30		3 6	0			8	0				
10:45		6	1	17	6	8 9	1	41	3	58	9
11:00		7	1	• •	J	10	1		Ü		Ü
11:15		6	0			15	1				
11:30		4	1			12	1				
11:45		7	0	24	2	15	1	52	4	76	6
											004
Total		209	380	209	380	464	614	464	614	673	994
Combined		58	39	58	9	10)78	10	78	16	67
Total											
AM Peak	-	07:15	-	-	-	07:15	-	-	-	-	-
Vol.	-	111	-	-	-	269	-	-	-	-	-
P.H.F.		0.631				0.606					
PM Peak	-	-	02:30	-	-	-	02:00	-	-	-	-
Vol.	-	-	130	-	-	-	125	-	-	-	-
P.H.F.			0.793				0.801				
			00				2.001				
Percentag											
e		35.5%	64.5%			43.0%	57.0%				
ADT/AADT	^	DT 1,667	Δ	ADT 1,667							
וערערון	,	ווטו, ווטו	^	וטו,ויוטו,							

Appendix F

Count Data for Comparison



BALOUR N-O MELBA

AM Period	NB		SB	El	3 W	/B	PM Period	NB		SB	E	B W	В	
00:00	6		5				12:00	58		48				
00:15	0		4				12:15	62		70				
00:30	1		0				12:30	59		44				
00:45	2	9	2	11		20	12:45	65	244	46	208			452
01:00	2		1				13:00	56		57				
01:15	2		3				13:15	46		64				
01:30	1		0				13:30	95		70				
01:45	0	5	3	7		12	13:45	72	269	49	240			509
02:00	2		1				14:00	78		62				
02:15	0		1				14:15	86		90				
02:30	1		1				14:30	88		66				
02:45	1	4	1	4		8	14:45	88	340	89	307			647
03:00	0		0				15:00	76		108				
03:15	1		1				15:15	152		96				
03:30	0		2				15:30	108		79				
03:45	0	1	1	4		5	15:45	64	400	73	356			756
04:00	3		0				16:00	95		73				
04:15	2		2				16:15	74		65				
04:30	5		0				16:30	83		66				
04:45	6	16	0	2		18	16:45	74	326	55	259			585
05:00	4		1				17:00	82		86				
05:15	2		2				17:15	82		85				
05:30	6		3				17:30	83		64				
05:45	14	26	8	14		40	17:45	85	332	88	323			655
06:00	16		13				18:00	71		71				
06:15	25		14				18:15	73		90				
06:30	23		7				18:30	69		70				
06:45	36	100	28	62		162	18:45	63	276	45	276			552
07:00	61		67				19:00	65		47				
07:15	105		125				19:15	56		31				
07:30	117		159				19:30	43		42				
07:45	99	382	115	466		848	19:45	35	199	37	157			356
08:00	63		60				20:00	41		29				
08:15	49		90				20:15	33		38				
08:30	66		73				20:30	35		40				
08:45	89	267	74	297		564	20:45	25	134	45	152			286
09:00	64		65				21:00	23		43				
09:15	56		44				21:15	14		19				
09:30	55		47				21:30	14		20				
09:45	49	224	41	197		421	21:45	14	65	12	94			159
	44		43			121		10		16	. ,			,
10:00 10:15	44		43				22:00 22:15	7		10				
10:15	48		34				22:15	8		11				
10.30	40	175	3 4 47	164		339	22:45	8	33	8	45			78
		170		107		337			55		10			7 0
11:00	61 50		39 54				23:00	5		9				
11:15	58 83		54 42				23:15	/		8 2				
11:30 11:45	83 57	259	42 80	215		474	23:30 23:45	5 2	19	2 4	23			42
	JI	209	00	∠ I ∪			23.43		17	4	۷.			
Total Vol.		1468		1443		2911			2637		2440			5077
												Daily Totals		
									NB		SB	EB	WB	Combined
									4105		3883			7988
					AM		_					PM		
Split %		50.4%)	49.6%		36.4%			51.9%)	48.1%			63.6%
Peak Hour		07:15		07:00		07:00			14:45		14:45			14:45
Volume P.H.F.		384 0.82		466 0.73		848 0.77			424 0.77		372 0.86			796 0.80
		1101		(1 /)		U. / /					(1()()			U.OU

PACIFIC TECHNICAL DATA, LLC

0.77

0.77

0.86

0.82

0.73

P.H.F.

0.80

Counts Unlimited, Inc. PO Box 1178

City of Encinitas Balour Drive B/ Encinitas Boulevard - Melba Road 24 Hour Directional Volume Count

Corona, CA 92878
Phone: (951) 268-6268
email: counts@countsunlimited.com

ECN002 Site Code: 143-21458

12:00 1 53 2 57 12:15 1 71 4 65 12:30 0 57 0 62 12:45 2 82 4 263 1 61 7 245 11 508 01:00 2 63 1 61 7 245 11 508 01:30 0 60 60 1 58 0 101:30 0 60 1 58 0 101:30 0 60 0 78 0 78 0 78 0 78 0 231 13 509 0	Start Time	31-Aug-21 Tue	North Morning	bound Afternoon	Hour ⁻ Morning	Totals Afternoon	South Morning	bound Afternoon	Hour Morning	Totals Afternoon	Combine Morning	d Totals Afternoon
12:15	12:00	rue			Worming	Alterioon			Worning	Aitemoon	Worming	Aitemoon
12:30			-	71			4					
12-45												
01:00					4	263			7	245	11	508
01:30	01:00		2				1	48				
0145 3 61 6 278 2 63 7 231 13 509 02:00 0 58 0 78 0 78 0 02:15 1 62 1 1 101 02:30 0 90 0 90 02:45 1 1 148 2 358 0 122 1 441 3 799 03:15 0 157 0 63 03:17 0 62 1 62 1 62 1 62 1 62 1 62 1 62 1 62			1	94			3					
02:00 0 58 0 78 0 78 0 78 0 78 0 78 0 78 0								58				
02:15					6	278	2		7	231	13	509
02:30												
03:45			-									
03:00					_							
03:15					2	358			1	441	3	799
03:30												
03:45												
04:00			2		7	400		62	2	200	10	700
04:15					,	490			3	200	10	700
04:30												
04.45	04.13 04:30		5									
05:00 2 101 2 95 05:15 7 81 3 3 73 05:30 7 91 9 62 23 280 53 625 06:00 15 67 13 68 13 52 60 63 63 63 37 625 06:30 33 53 101 238 44 42 89 209 190 447 07:00 37 36 63 37 63 37 70 715 73 36 63 37 70 715 73 36 63 37 70 715 73 36 74 18 42 44 42 89 209 190 447 07:30 103 23 72 170 30 14 952 271 271 280 22 271 280 28 44 49 98			3		11	364			. 2	309	13	673
05:30					• • •	004		95		000	10	070
05:30	05:15						3	73				
06:45							9					
06:00	05:45		14		30	345	9	50	23	280	53	625
06:30			15	67				68				
06:45	06:15		13	63			13	52				
07:00							19	47				
07:15					101	238		42	89	209	190	447
07:30												
07:45												
08:00					270	104			E72	117	050	074
08:15 08:30 08:45 08:30 08:45 74 18 457 68 58 24 349 98 806 166 09:00 57 33 444 19 09:15 62 15 09:30 59 17 40 10:00 44 9 36 15 10:30 10:15 55 5 40 11:00 57 2 38 55 19 09:30 59 17 40 10 10 10 11 10:15 55 5 5 40 11 11:15 66 00 7 11:15 66 00 250 2 11:15 66 00 250 2 11:45 66 00 250 2 11:45 66 00 250 2 11:45 66 00 250 2 145 00 205 8 455 10 Total Combined Total AM Peak - 07:30 07:15				29	3/9	124			5/3	147	952	2/1
08:30				10								
08:45	08:30						55					
09:00					457	68	58		349	98	806	166
09:15 62 15 47 17 09:30 59 17 40 10 09:45 59 7 237 72 39 16 170 62 407 134 10:00 44 9 36 15 36 15 36 15 36 15 36 15 36 15 36 15 36 15 36 15 36 15 36 15 36 15 36 15 36 15 36 383 60 36 383 60 36 383 60 383 60 383 60 36 383 60 383 60 383 60 383 60 383 60 383 60 383 60 383 60 383 60 383 60 383 60 383 60 383 40 382 432 432 432 432				33	101	00	44		0.10	00	000	.00
09:30 59 17 237 72 39 16 170 62 407 134 09:45 59 7 237 72 39 16 170 62 407 134 10:00 444 9 36 15 40 11 10 60 7 60 7 38 5 5 60 7 60 38 5 176 36 383 60 383 60 60 7 2 52 5 5 176 36 383 60 60 7 2 52 5 176 36 383 60 60 60 7 2 50 2 176 36 383 60 8 45 11 11 11 11 11 11 15 66 0 250 2 45 0 205 8 455 10 10 11 11 <t< td=""><td></td><td></td><td>62</td><td>15</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>			62	15								
09:45 59 7 237 72 39 16 170 62 407 134 10:00 444 9 36 15 40 11 55 55 5 40 11 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 7 60 60 7 60 60 60 7 60				17								
10:15 55 5 40 11 11 10:30 60 7 338 5 176 36 383 60 11:00 57 2 2 62 5 176 36 383 60 11:00 57 2 52 5 5 176 36 383 60 11:15 65 0 50 2 58 1 20 8 455 10 11:30 62 0 250 2 45 0 205 8 455 10 11:45 66 0 250 2 45 0 205 8 455 10 Total 1691 2634 1691 2634 1605 2354 1605 2354 3296 4988 Combined Total 4325 4325 3959 3959 3959 8284 AM Peak - 07:30 07:15 0 - 0 - 0.00 - 0.00 - 0.00 - 0.00 - 0.00 - 0.00 - 0.00 - 0.00 - 0.00	09:45		59		237	72	39	16	170	62	407	134
10:30 60 7 48 3 207 24 62 5 176 36 383 60 11:00 57 2 5 5 5 176 36 11:00 577 2 52 5 5 11:15 65 0 50 2 11:15 65 0 0 50 2 11:30 62 0 0 58 1 1 50 58 1 1 50 50 2 11:45 66 0 0 250 2 45 0 205 8 455 10 50 10:00 50 50 50 50 50 50 50 50 50 50 50 50 5	10:00			9			36					
10:45			55	5			40					
11:00	10:30		60	7			38	5				
11:15 65 0 50 2 11:30 62 0 250 2 45 0 205 8 455 10 Total 1691 2634 1691 2634 1605 2354 1605 2354 3296 4988 Combined Total 4325 4325 3959 3959 3959 8284 AM Peak - 07:30 07:15	10:45		48	3	207	24	62	5	176	36	383	60
11:30 62 0 250 2 45 0 205 8 455 10 Total 1691 2634 1691 2634 1605 2354 1605 2354 3296 4988 Combined Total 4325 4325 3959 3959 3959 8284 AM Peak - 07:30 - - 07:15 - - - - - Vol. - 571 - - 680 - - - - - P.H.F. 0.696 0.895 -								5				
11:45 66 0 250 2 45 0 205 8 455 10 Total 1691 2634 1691 2634 1605 2354 3296 4988 Combined Total 4325 4325 3959 3959 3959 8284 AM Peak - 07:30 - - 07:15 - - - - - Vol. - 571 - - 680 -												
Total 1691 2634 1691 2634 1605 2354 1605 2354 3296 4988 Combined Total 4325 4325 3959 3959 8284 AM Peak - 07:30 07:15					250	2			205	Ω	155	10
Combined Total 4325 4325 3959 3959 8284 AM Peak - 07:30 07:15												
Total AM Peak - 07:30 07:15												
Vol. - 571 - - 680 - <			432	25	432	25	398	59	39	59	828	i4
P.H.F. 0.696 PM Peak 02:45 02:15	AM Peak	-	07:30	-	-	-	07:15	-	-	-	-	-
PM Peak 02:45 02:15	Vol.	-		-	-	-		-	-	-	-	-
Vol. - - 582 - - - 458 - - - - - PH.F. 0.846 0.818 Percentag 39.1% 60.9% 40.5% 59.5%												
P.H.F. 0.846 0.818 Percentag e 39.1% 60.9% 40.5% 59.5%		-			-	-	-		-	-	-	-
Percentag		-	-		-	-	-		-	-	-	-
e 39.1% 60.9% 40.5% 59.5%	P.H.F.			0.846				0.818				
e 39.1% 60.9% 40.5% 59.5%	Percentag											
	•		39.1%	60.9%			40.5%	59.5%				
			ADT 8,284	Α	ADT 8,284							

Appendix G

Existing Intersection LOS Worksheets



	٠	→	*	•	+	4	1	†	/	/	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	171	36	2	17	124	35	10	164	7	21	165	173
Future Volume (vph)	171	36	2	17	124	35	10	164	7	21	165	173
Confl. Peds. (#/hr)	240		15	15		240	1		25	25		1
Confl. Bikes (#/hr)			60			40			10			30
Peak Hour Factor	0.89	0.89	0.89	0.76	0.76	0.76	0.75	0.75	0.75	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												





1: Balour Dr & Melba Rd

Intersection												
Intersection Delay, s/veh	16.1											
Intersection LOS	C											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	171	36	2	17	124	35	10	164	7	21	165	173
Future Vol, veh/h	171	36	2	17	124	35	10	164	7	21	165	173
Peak Hour Factor	0.89	0.89	0.89	0.76	0.76	0.76	0.75	0.75	0.75	0.86	0.86	0.86
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	192	40	2	22	163	46	13	219	9	24	192	201
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	14.7			13.9			13.9			19.3		
HCM LOS	В			В			В			C		
Lane		NBLn1	EBLn1	WBLn1	SBLn1							
Vol Left, %		6%	82%	10%	6%							
Vol Thru, %		91%	17%	70%	46%							
Vol Right, %		4%	1%	20%	48%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		181	209	176	359							
LT Vol		10	171	17	21							
Through Vol		164	36	124	165							
RT Vol	``	7	2	35	173							
Lane Flow Rate		241	235	232	417							
Geometry Grp		1	1	1	1							
Degree of Util (X)		0.422	0.432	0.411	0.662							
Departure Headway (Hd)		6.302	6.619	6.388	5.713							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Cap		570	542	563	630							
Service Time		4.359	4.675	4.444	3.76							
HCM Lane V/C Ratio		0.423	0.434	0.412	0.662							
HCM Control Delay		13.9	14.7	13.9	19.3							
HCM Lane LOS		В	В	В	C							
HCM 95th-tile Q		2.1	2.2	2	4.9							

AM Existing 3: Crest Dr & Melba Rd

	٦	•	1	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Traffic Volume (vph)	97	4	17	35	50	216
Future Volume (vph)	97	4	17	35	50	216
Confl. Peds. (#/hr)	9		4			4
Confl. Bikes (#/hr)		10				30
Peak Hour Factor	0.70	0.70	0.57	0.57	0.69	0.69
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Intersection Summary						



Intersection							
Intersection Delay, s/veh	9.5					<u> </u>	
Intersection LOS	A						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	N/	EDIT	HUL	4	1	ODIL	
Traffic Vol, veh/h	97	4	17	€ 1	50	216	
Future Vol, veh/h	97	4	17	35	50	216	
Peak Hour Factor	0.70	0.70	0.57	0.57	0.69	0.69	
Heavy Vehicles, %	2	2	2	2	2	2	
Mymt Flow	139	6	30	61	72	313	
Number of Lanes	1	0	0	1	1	0	
				<u> </u>		J	
Approach	EB		NB		SB		
Opposing Approach			SB		NB		
Opposing Lanes	0		1		1		
Conflicting Approach Left	SB		EB				
Conflicting Lanes Left	1		1		0		
Conflicting Approach Right	NB		•		EB		
Conflicting Lanes Right	1		0		1		
HCM Control Delay	9.4		8.4		9.8		
HCM LOS	A		A		A		
Lane		NBLn1	EBLn1	SBLn1			
Vol Left, %		33%	96%	0%			
Vol Thru, %		67%	0%	19%			
Vol Right, %		0%	4%	81%			
Sign Control		Stop	Stop	Stop			
Traffic Vol by Lane		52	101	266			
LT Vol		17	97	0			
Through Vol		35	0	50			
RT Vol		0	4	216			
Lane Flow Rate		91	144	386			
Geometry Grp		1	1	1			
Degree of Util (X)		0.121	0.204	0.422			
Departure Headway (Hd)		4.759	5.086	3.938			
Convergence, Y/N		Yes	Yes	Yes			
Cap		753	704	914			
Service Time		2.791	3.13	1.958			
HCM Lane V/C Ratio		0.121	0.205	0.422			
HCM Control Delay		8.4	9.4	9.8			
HCM Lane LOS		A	A	A			
HCM 95th-tile Q		0.4	0.8	2.1			

	•	→	•	•	+	4	•	†	/	\	+	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	133	24	10	7	54	24	9	236	7	12	148	111
Future Volume (vph)	133	24	10	7	54	24	9	236	7	12	148	111
Confl. Peds. (#/hr)	12		3	3		12			5	12		
Confl. Bikes (#/hr)			10			10			5			5
Peak Hour Factor	0.85	0.85	0.85	0.82	0.82	0.82	0.93	0.93	0.93	0.89	0.89	0.89
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Intersection Summary												



1: Balour Dr & Melba Rd

Intersection												
Intersection Delay, s/veh	11.4											
Intersection LOS	В											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	133	24	10	7	54	24	9	236	7	12	148	111
Future Vol, veh/h	133	24	10	7	54	24	9	236	7	12	148	111
Peak Hour Factor	0.85	0.85	0.85	0.82	0.82	0.82	0.93	0.93	0.93	0.89	0.89	0.89
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	156	28	12	9	66	29	10	254	8	13	166	125
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	11.4			9.8	_		11.7			11.6		
HCM LOS	В			A			В			В		
							· ·					
Lane		NBLn1	EBLn1	WBLn1	SBLn1							
Vol Left, %		4%	80%	8%	4%							
Vol Thru, %		94%	14%	64%	55%							
Vol Right, %		3%	6%	28%	41%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		252	167	85	271							
LT Vol		9	133	7	12							
Through Vol		236	24	54	148							
RT Vol		7	10	24	111							
Lane Flow Rate		271	196	104	304							
Geometry Grp		1	1	1	1							
Degree of Util (X)		0.395	0.312	0.162	0.422							
Departure Headway (Hd)		5.244	5.723	5.638	4.985							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Cap		686	627	634	722							
Service Time		3.278	3.765	3.686	3.018							
HCM Lane V/C Ratio		0.395	0.313	0.164	0.421							
HCM Control Delay		11.7	11.4	9.8	11.6							
HCM Lane LOS		В	D	Α	D							
HCM 95th-tile Q		D	В	A	В							

PM Existing 3: Crest Dr & Melba Rd

	*	•	•	†		4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Traffic Volume (vph)	42	2	8	26	38	80
Future Volume (vph)	42	2	8	26	38	80
Confl. Peds. (#/hr)			1			1
Confl. Bikes (#/hr)		5				5
Peak Hour Factor	0.79	0.79	0.71	0.71	0.82	0.82
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%			0%	0%	
Shared Lane Traffic (%)						
Intersection Summary						



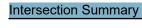
Intersection							
Intersection Delay, s/veh	7.5						
Intersection LOS	A						
Novement	EBL	EBR	NBL	NBT	SBT	SBR	
ane Configurations	W	LDI	NDL	4	1>	ODI	
raffic Vol, veh/h	42	2	8	26	38	80	
iture Vol, veh/h	42	2	8	26	38	80	
ak Hour Factor	0.79	0.79	0.71	0.71	0.82	0.82	
avy Vehicles, %	2	2	2	2	2	2	
vmt Flow	53	3	11	37	46	98	
mber of Lanes	1	0	0	1	1	0	
				•			
proach	EB		NB SB		SB NB		
oposing Approach oposing Lanes	0		3B		NB 1		
osing Lanes oflicting Approach Left	SB		EB				
ifflicting Lanes Left	3B 1		ED 1		0		
flicting Approach Right	NB				EB		
flicting Lanes Right	ND 1		0		ED 1		
l Control Delay	7.8		7.5		7.3		
I LOS	7.0 A		7.3 A		7.5 A		
II LUJ	- A		- А		M		
e		NBLn1	EBLn1	SBLn1			
eft, %		24%	95%	0%			
Thru, %		76%	0%	32%			
Right, %		0%	5%	68%			
n Control		Stop	Stop	Stop			
ffic Vol by Lane		34	44	118			
/ol		8	42	0			
ough Vol		26	0	38			
/ol		0	2	80			
e Flow Rate		48	56	144			
metry Grp		1	1	1			
ree of Util (X)		0.056	0.069	0.146			
arture Headway (Hd)		4.189	4.428	3.662			
vergence, Y/N		Yes	Yes	Yes			
. •		849	803	973			
vice Time		2.243	2.487	1.711			
A Control Polon		0.057	0.07	0.148			
M Control Delay		7.5	7.8	7.3			
M Lane LOS		A	A	A			
M 95th-tile Q		0.2	0.2	0.5			

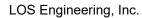
Appendix H

Existing + Project Intersection LOS Worksheets



	۶	→	•	•	+	•	1	†	~	/	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	171	37	2	22	128	40	10	164	9	23	165	173
Future Volume (vph)	171	37	2	22	128	40	10	164	9	23	165	173
Confl. Peds. (#/hr)	240		15	15		240	1		25	25		1
Confl. Bikes (#/hr)			60			40			10			30
Peak Hour Factor	0.89	0.89	0.89	0.76	0.76	0.76	0.75	0.75	0.75	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												





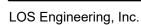
Intersection Delay, s/veh 16.8 Intersection LOS C	Intersection		
Intersection LOS C	Intersection Delay, s/veh	16.8	
	Intersection LOS	С	

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	171	37	2	22	128	40	10	164	9	23	165	173
Future Vol, veh/h	171	37	2	22	128	40	10	164	9	23	165	173
Peak Hour Factor	0.89	0.89	0.89	0.76	0.76	0.76	0.75	0.75	0.75	0.86	0.86	0.86
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	192	42	2	29	168	53	13	219	12	27	192	201
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB		-	NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Righ	nt NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	15			14.7			14.4			20.4		
HCM LOS	В			В			В			С		

Lane	NBLn1	EBLn1\	WBLn1	SBLn1	
Vol Left, %	5%	81%	12%	6%	
Vol Thru, %	90%	18%	67%	46%	
Vol Right, %	5%	1%	21%	48%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	183	210	190	361	
LT Vol	10	171	22	23	
Through Vol	164	37	128	165	
RT Vol	9	2	40	173	
Lane Flow Rate	244	236	250	420	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.435	0.441	0.448	0.679	
Departure Headway (Hd)	6.414	6.727	6.449	5.819	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	559	533	558	621	
Service Time	4.479	4.793	4.515	3.874	
HCM Lane V/C Ratio	0.436	0.443	0.448	0.676	
HCM Control Delay	14.4	15	14.7	20.4	
HCM Lane LOS	В	В	В	С	
HCM 95th-tile Q	2.2	2.2	2.3	5.2	

2: Melba Rd & Project Driveway

	•	→	←	•	-	1	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Traffic Volume (vph)	5	64	176	1	1	14	
Future Volume (vph)	5	64	176	1	1	14	
Confl. Peds. (#/hr)	10	O I	170	10	10	10	
Confl. Bikes (#/hr)				10		10	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Growth Factor	100%	100%	100%	100%	100%	100%	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	
Bus Blockages (#/hr)	0	0	0	0	0	0	
Parking (#/hr)							
Mid-Block Traffic (%)		0%	0%		0%		
Shared Lane Traffic (%))						
Intersection Summary							



Intersection						
Int Delay, s/veh 0.7	7					
Movement EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	4	ĵ.		W		
Traffic Vol, veh/h		176	1	1	14	
Future Vol, veh/h	64	176	1	1	14	
Conflicting Peds, #/hr10	0	0	10	10	10	
Sign Control Free	Free	Free	Free	Stop	Stop	
	- None	-	None	-	None	
otorago Longan		-	-	0	-	
Veh in Median Storage;		0	-	0	-	
O1440, 70	- 0	0	-	0	-	
Peak Hour Factor 92		92	92	92	92	
Heavy Vehicles, % 2		2	2	2	2	
Mvmt Flow 5	5 70	191	1	1	15	
Major/Minor Major1		1ajor2	M	linor2		
Conflicting Flow All 202	2 0	-	0	292	212	
otago i		-	-	202	-	
Stage 2		-	-	90	-	
Critical Hdwy 4.12	2 -	-	-		6.22	
· · · · · · · · · · · · · · · ·		-	-	5.42	-	
Critical Hdwy Stg 2		-		5.42	-	
Follow-up Hdwy 2.218		-		3.518		
Pot Cap-1 Maneuve870) -		-	699	828	
Stage 1	-	-		832	-	
Stage 2	-	-		934	-	
Platoon blocked, % Mov Cap-1 Maneuvlæ59	_			685	814	
Mov Cap-1 Maneuver		_		685		
Stage 1		-	-	822		
01 0	_		_	927		
Otage 2	_		_	521	_	
Approach)	VAC		CD		
Approach EE		WB		SB		
HCM Control Delay, §.6)	0		9.6 A		
I IOWI LOS				А		
			14/5-	===		
Minor Lane/Major Mvmt		FBI	WBT	WBHS		
Capacity (veh/h)	1359	-	-	-	804	
HCM Control Doloy (a)	0.004	-	-		0.02	
HCM Control Delay (s) HCM Lane LOS	7.7	0	-	-	9.6	
HCM 95th %tile Q(veh)	A 0	A -	-	-	0.1	
HOW SOUT WHIE Q(VEH)	U	-	-	-	0.1	

	۶	•	4	†	↓	4	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Traffic Volume (vph)	98	4	17	35	50	217	
Future Volume (vph)	98	4	17	35	50	217	
Confl. Peds. (#/hr)	9		4			4	
Confl. Bikes (#/hr)		10				30	
Peak Hour Factor	0.70	0.70	0.57	0.57	0.69	0.69	
Growth Factor	100%	100%	100%	100%	100%	100%	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	
Bus Blockages (#/hr)	0	0	0	0	0	0	
Parking (#/hr)							
Mid-Block Traffic (%)	0%			0%	0%		
Shared Lane Traffic (%)							
Intersection Summary							



Intersection							
Intersection Delay, s/veh	9.5						
Intersection LOS	Α						
	ED!	EDD	NDI	NDT	ODT	000	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			र्	f»	~	
Traffic Vol, veh/h	98	4	17	35	50	217	
Future Vol, veh/h	98	4	17	35	50	217	
Peak Hour Factor	0.70	0.70	0.57	0.57	0.69	0.69	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	140	6	30	61	72	314	
Number of Lanes	1	0	0	1	1	0	
Approach	EB		NB		SB		
Opposing Approach			SB		NB		
Opposing Lanes	0		1		1		
Conflicting Approach Left	t SB		EB				
Conflicting Lanes Left	1		1		0		
Conflicting Approach Rig	ht NB				EB		
Conflicting Lanes Right	1		0		1		
HCM Control Delay	9.5		8.5		9.8		
HCM LOS	Α		Α		А		
Lane		NBLn1	EBLn1				
Vol Left, %		33%	96%	0%			
Vol Thru, %		67%	0%	19%			
Vol Right, %		0%	4%	81%			
Sign Control		Stop	Stop	Stop			
Traffic Vol by Lane		52	102	267			
LT Vol		17	98	0			
Through Vol		35	0	50			
RT Vol		0	4	217			
Lane Flow Rate		91	146	387			
Geometry Grp		1	1	1			
Degree of Util (X)		0.121	0.206	0.424			
Departure Headway (Hd)		4.765	5.092				
Convergence, Y/N		Yes	Yes	Yes			
Cap		752	704	913			
Service Time		2.797	3.134	1.962			
Service Time HCM Lane V/C Ratio		0.121	0.207	0.424			
Service Time HCM Lane V/C Ratio HCM Control Delay		0.121 8.5	0.207 9.5	0.424 9.8			
Service Time HCM Lane V/C Ratio		0.121	0.207	0.424			

	۶	→	•	•	+	•	1	†	~	/	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	133	28	10	10	55	27	9	236	14	18	148	111
Future Volume (vph)	133	28	10	10	55	27	9	236	14	18	148	111
Confl. Peds. (#/hr)	12		3	3		12			5	12		
Confl. Bikes (#/hr)			10			10			5			5
Peak Hour Factor	0.85	0.85	0.85	0.82	0.82	0.82	0.93	0.93	0.93	0.89	0.89	0.89
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%))											

Intersection Summary



Intersection		
Intersection Delay, s/veh	11.7	
Intersection LOS	В	

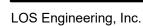
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	133	28	10	10	55	27	9	236	14	18	148	111
Future Vol, veh/h	133	28	10	10	55	27	9	236	14	18	148	111
Peak Hour Factor	0.85	0.85	0.85	0.82	0.82	0.82	0.93	0.93	0.93	0.89	0.89	0.89
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	156	33	12	12	67	33	10	254	15	20	166	125
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	t SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Rig	ht NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	11.6			10			12			12		
HCM LOS	В			Α			В			В		

Lane	NBLn1	EBLn1\	WBLn1	SBLn1	
Vol Left, %	3%	78%	11%	6%	
Vol Thru, %	91%	16%	60%	53%	
Vol Right, %	5%	6%	29%	40%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	259	171	92	277	
LT Vol	9	133	10	18	
Through Vol	236	28	55	148	
RT Vol	14	10	27	111	
Lane Flow Rate	278	201	112	311	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.409	0.324	0.178	0.437	
Departure Headway (Hd)	5.293	5.79	5.704	5.059	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	680	620	627	710	
Service Time	3.337	3.836	3.757	3.101	
HCM Lane V/C Ratio	0.409	0.324	0.179	0.438	
HCM Control Delay	12	11.6	10	12	
HCM Lane LOS	В	В	Α	В	
HCM 95th-tile Q	2	1.4	0.6	2.2	

	•	→	←	•	\	1
--	---	----------	----------	---	----------	---

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Traffic Volume (vph)	17	43	85	2	1	7
Future Volume (vph)	17	43	85	2	1	7
Confl. Peds. (#/hr)	10			10	10	10
Confl. Bikes (#/hr)				10		10
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						

Intersection Summary



Intersection						
Int Delay, s/veh 1	.3					
Movement EE	BL EBT	WBT	WBR		SBR	
Lane Configurations	4	₽		W		
Traffic Vol, veh/h	17 43		2	1	7	
Future Vol, veh/h	17 43	85	2	1	7	
Conflicting Peds, #/hr1	10 0	0	10	10	10	
Sign Control Fre	e Free	Free	Free	Stop	Stop	
RT Channelized	- None	-	None	-	None	
Storage Length		-	-	0	-	
Veh in Median Storage	e,# 0	0	-	0	-	
Grade, %	- 0	0	-	0	-	
Peak Hour Factor 9	92 92	92	92	92	92	
Heavy Vehicles, %	2 2	2	2	2	2	
Mvmt Flow 1	18 47	92	2	1	8	
Major/Minor Major	r1 N	/lajor2	M	inor2		
Conflicting Flow All 10	04 0	-	0	196	113	
Stage 1		-	-	103	_	
Stage 2		-	-	93	-	
Critical Hdwy 4.1	l2 -	-	-	6.42	6.22	
Critical Hdwy Stg 1		-	-	5.42	-	
Critical Hdwy Stg 2		-	-	5.42	-	
Follow-up Hdwy 2.21	I8 -	-	- (3.518	3.318	
Pot Cap-1 Maneuve48	38 -	-	-	793	940	
Stage 1	-	-	-	921	-	
Stage 2	- (-	-	-	931	-	
Platoon blocked, %	-	_				
Mov Cap-1 Maneu งใฝ ั <i>ก</i> ั		_		770	924	
Mov Cap-2 Maneuver		-	-	770	-	
Stage 1		-	-	902	-	
Stage 2		7	-	924	-	
Approach E	В	WB		SB		
HCM Control Delay, থ	.1	0		9		
HCM LOS				Α		
Minor Lane/Major Mvn			WBT	WBRS	BLn1	
Capacity (veh/h)	1476		-	-	901	
HCM Lane V/C Ratio	0.013		-	-		
HCM Control Delay (s			-	-	9	
HCM Lane LOS	Α		-	-	Α	
HCM 95th %tile Q(veh	1) 0	-	-	-	0	

	•	•	1	†	↓	4	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Traffic Volume (vph)	43	2	8	26	38	82	
Future Volume (vph)	43	2	8	26	38	82	
Confl. Peds. (#/hr)			1			1	
Confl. Bikes (#/hr)		5				5	
Peak Hour Factor	0.79	0.79	0.71	0.71	0.82	0.82	
Growth Factor	100%	100%	100%	100%	100%	100%	
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	
Bus Blockages (#/hr)	0	0	0	0	0	0	
Parking (#/hr)							
Mid-Block Traffic (%)	0%			0%	0%		
Shared Lane Traffic (%)						

Intersection Summary



Intersection								
Intersection Delay, s/veh	7.5							
Intersection LOS	Α							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	W			र्स	₽			
Traffic Vol, veh/h	43	2	8	26	38	82		
Future Vol, veh/h	43	2	8	26	38	82		
Peak Hour Factor	0.79	0.79	0.71	0.71	0.82	0.82		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	54	3	11	37	46	100		
Number of Lanes	1	0	0	1	1	0		
Approach	EB		NB		SB			
Opposing Approach			SB		NB			
Opposing Lanes	0		1		1			
Conflicting Approach Left	SB		EB					
Conflicting Lanes Left	1		1		0			
Conflicting Approach Righ	nt NB				EB			
Conflicting Lanes Right	1		0		1			
HCM Control Delay	7.8		7.5		7.4			
HCM LOS	Α		Α		Α			
Lane	N	IBLn1	EBLn1	SBLn1			· ·	
Vol Left, %		24%	96%	0%				
Vol Thru, %		76%	0%	32%				
Vol Right, %		0%	4%	68%				
Sign Control		Stop	Stop	Stop				
Traffic Vol by Lane		34	45	120				
LT Vol		8	43	0				
Through Vol		26	0	38				
RT Vol	,	0	2	82				
Lane Flow Rate		48	57	146				
Geometry Grp		1	1	1				
Degree of Util (X)		0.056	0.07	0.149				
Departure Headway (Hd)		4.193	4.433	3.661				
Convergence, Y/N		Yes	Yes	Yes				
Cap		848	802	973				
Service Time		2.248	2.494	1.71				
HCM Lane V/C Ratio		0.057	0.071	0.15				
HCM Control Delay		7.5	7.8	7.4				
HCM Lane LOS		Α	Α	Α				
HCM 95th-tile Q		0.2	0.2	0.5				